Investigating new materials in the context of analog-physical models

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ARTICLE INFO

Article history:
Received 7 February 2012
Received in revised form 19 September 2012
Accepted 20 September 2012
Available online 5 October 2012

Keywords:
Analog modeling
Sand mixtures
Angle of internal friction
Shear strength
Rheology

ABSTRACT

To broaden the availability of granular materials that are suitable for the analog modeling of upper crustal deformation, we investigated the mechanical behaviors of pure quartz sand and two sand mixtures (quartz sand–powdered barite and quartz sand mica crystals) using ring-shear tests and simple convergent sandbox experiments. The ring-shear test results indicate that the three materials have similar peak friction angles (between 39.25° and 36.02°), but the magnitude of the shear strain and the shear strength required to cause their failure are different. The differences between the analog models are identified by distinct fault kinematics and different grain flows, which are primarily related to differences in the plastic elasto-frictional rheology. We conclude that the use of the quartz–mica mixture, which showed the strongest distributed (plastic) deformation, can improve analog models where different materials are required to simulate crystalline basement (sand) and supracrustal rocks (sand mica mixture). This is a common situation in extension and inversion tectonics, such as, for example, in inversion tectonics, when a granitic basement block acts as a buttress.

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1. Introduction

The materials that are most frequently used for the physical modeling of brittle deformation are dry quartz sand and wet clay. The frictional behavior of these materials is similar, but the cohesive strength of sand is much less (but not negligible) than that of wet clay (Withjack et al., 2007). Several studies concerning the mechanical properties of dry sand and wet clay have been published (e.g., Mandl, 1988; Krantz, 1991; Schellart, 2000; Eisenstadt and Sims, 2005; Withjack and Schlische, 2006; Panien et al., 2006). These studies suggest that the frictional characteristics of sand and clay in a normal gravity field obey the Coulomb Failure Criterion (Byerlee, 1978):

\[ \tau = C_0 + \mu \sigma_N \]  

where \( \tau \) and \( \sigma_N \) are, respectively, the shear and normal stresses, \( C_0 \) is the cohesive strength, and \( \mu \) is the coefficient of internal friction.

Lohrmann et al. (2003) have shown that quartz sand is characterized by an elastic/frictional plastic behavior in which strain hardening precedes faulting, after which strain softening occurs until stable stress values are obtained. This behavior simulates that of brittle rocks. However, the use of dry sand and wet clay as analog materials to simulate the upper crust is an oversimplification, and it does not satisfy situations where the simulation of rock units with different rheological behaviors are called for. To address this problem, several researchers (e.g., Schellart, 2000; Schöpfer and Streyer, 2001; Rossi and Storti, 2003; Panien et al., 2005) have investigated new, alternative analog materials. Schöpfer and Streyer (2001) used different sand types (sifted natural dry quartz with a rough surface and low sphericity, the same quartz without clay mineral fractions and a coarser-grained industrially produced sand with smooth grain surfaces and high-sphericity) in strike-slip fault models, and they have shown that the geometry of shear zones does not change with the varying thickness of the sand pack but that it instead changes with grain properties. Gabrielsen and Clausen (2001) used plaster of Paris to enhance the delicate details of extensional tectonics. Lohrmann et al. (2003) described changes in the geometry, kinematics and dynamics of convergent sandbox experiments using sand with different grain-size distributions and different shear apparatus filling techniques. Rossi and Storti (2003) employed fine-grained, high-sphericity, low-density aluminum and siliceous hollow microspheres to simulate the mechanical behavior of ductile detachments within sand packs. They argued that an extremely fine-grained material provides more structural detail and better scaling than coarser-grained sand. Glass microbeads were utilized by several authors to simulate multilayered mechanical stratigraphy, weak sedimentary rocks or ductile shear zones (e.g., Turrini et al., 2001; Panien et al., 2006; Massoli et al., 2006; Yamada et al., 2006; Ravaglia et al., 2006; Yagupsky et al., 2008; Konstantinovskaya and Malavieille, 2011). This
material consists of highly spherical grains that have an angle of internal friction consistently lower than that of quartz sand. The aim of our work was to contribute to the study of new elastic and frictional plastic analog materials. Shortening experiments were conducted with three types of sands, the mechanical properties of which are characterized in detail. The study is not intended to represent any particular natural geological setting, but to provide an approach for the identification of materials suitable for differentiation among crystalline basement and supracrustal rocks in inversion tectonics or thick-skinned fold-and-thrust belts. Many sandbox experiments on basin inversion have been conducted to analyze hanging wall deformations using rigid footwalls (made of acrylic or wood) (e.g., McClay, 1989; Buchanan and McClay, 1992; Gomes et al., 2010). However, these rigid footwalls cannot deform as a basement rock might deform in nature. In addition, the high strength contrast between the rigid footwalls and the analog material influences the model results. We analyzed pure quartz sand and mixtures of sands with two properties of differing composition and shape: sand with powdered barite (2:1 by weight) and sand with mica crystals (14:1 by weight).

First, we used a ring-shear tester to examine the optimal ratio between sand and barite and sand and mica crystals. In addition, we analyzed the relationship between the frictional slope of 400 Pa of the quartz sand and two preparation techniques. Second, we measured the angle of the internal friction and the cohesion of the pure quartz sand (for ease of description, in the following, we refer to the pure quartz sand as sand) and the sand mixtures. Third, we applied three analog models to each of the three model materials and compared the results. Finally, we used the model results to improve our knowledge of the mechanical behavior of granular materials.

It was not our intention to monitor the strain history of the sand mixture models, but rather to compare the mechanical response of each of them with that of the sand model. Accordingly to Richard and Krantz (1991) and Withjack et al. (2007), the primary deformation mechanism in sand is localized deformation (e.g., failure), although distributed deformation is also important (e.g., Rutter, 1986). Distributed deformation causes strain without loss of continuity, commonly referred to as ductile behavior. In the following text, we refer to the distributed deformation process in the vertical and in the horizontal directions (as described by the above authors) as vertical grain flow or vertical ejection of the sand and as horizontal grain flow, respectively. The latter is sometimes referred as layer-parallel shortening, as used by Koyi (1995).

2. The mechanical properties of sand and sand mixtures

2.1. Materials and methods

The Schulze ring-shear tester (RST-XS) was developed in 1994 based on equipment that has been used in soil mechanics (rotational shear testers) since the 1930s. Among the simple shear apparatuses that are currently available, the RST-XS is notable because it works with low normal loads that are within the range of stresses applied to the model materials in sandboxes.

In the present study, the methodology for use of the RST-XS followed that described by Ellis et al. (2004) and Pani en et al. (2006). The measurements of the internal friction (and basal friction) angles and cohesion strength were performed with normal stresses, which ranged from 800 to 2400 Pa, in intervals of 400 Pa. The computer-controlled RST-XS recorded the shear stress in three different situations: at fault initiation (the peak strength), at fault reactivation (the second peak strength or the reactivation peak strength) and at fault sliding (the dynamic–stable strength); it was also used to generate stress–strain curves in diagrams of shear stress plotted as a function of time.

The values obtained by the RST-XS were used to produce a second diagram to compare the shear stress to the normal stress (Coulomb–Mohr diagram). The coefficient of the internal friction (and basal friction) was derived from the linear regression analysis of the diagram, whereas the cohesion was inferred by extrapolation along the straight Coulomb fracture envelope to its intersection with the shear stress axis.

In preliminary tests, we analyzed several ratios of sand mixtures to find a material that was characterized by a peak angle of internal friction similar to that of the sand. This was important because we were searching for materials that were only slightly different from sand (such as slightly less competent supracrustal rocks compared to infracrustal rocks). The tests revealed that the sand mixture ratios had no major influence on the peak angles of internal friction (for example, the sand mica mixtures of ratios 14:1 and 24:1, by weight, revealed a difference of only 2.03° in their peak angles of internal friction). Thus, the choice of the ratio (by weight) of our sand mixtures, sand with barite, 2:1, and sand with mica crystals, 14:1, has only impacted the shear strength and cohesion values.

Measurements of the frictional properties of sand were obtained using different preparation techniques following Krantz (1991), Schellart (2000) and Lohrmann et al. (2003). The annular cell of the ring-shear tester was filled by both sifting and pouring (and afterward, leveled by scraping), and different fill heights were examined. We analyzed the frictional properties obtained by pouring the analog material into the test cell, as pouring, in contrast to sifting, can prevent the loss of powdered barite during preparation. Moreover, to avoid any loss of the low-density mica crystals, especially during pouring, we poured this mixture directly from the edge of the annular cell (thus, the height was equal to zero). Finally, in one sample, a < 210 µm grain size was analyzed.

To minimize measurement errors, each test was repeated three times, and a total of 15 tests were performed for each sample. The statistical calculations provided R² correlation values (the square of the correlation coefficient) that were greater than 0.99 for all tests.

2.2. Results

2.2.1. Frictional properties of sand as a function of filling techniques

The experimental results presented in Table 1(a) reveal that the filling technique (sifting or pouring) is the only parameter that influences the angle of the internal friction of our sand. However, these variations occur only in the peak strength values, i.e., at failure (a localized shear zone). The peak friction (the internal friction angle (φ) derived from the peak strength) of sand sifted from a height of 20 cm is 41.47°, and pouring that sand from the same height resulted in an angle of 37.97°. The height and granulometry variations produced minimal differences of approximately 1° in the angle of peak internal friction.

The stress–strain curves of the shear stress plotted as a function of time for the sand sifted and poured from the same height are presented in Fig. 1. In the sifted sand, the peak strength is substantially higher than that of the poured sand, at values of 2254 Pa and 1996 Pa, respectively (for the highest normal load of 2400 Pa). The dynamic stable strength and the reactivation peak strength, however, vary only slightly. Consequently, the poured sand has a lower rate of strain softening (measured as the difference between the coefficient of peak friction and the dynamic–stable friction divided by the coefficient of the dynamic–stable friction) (Table 1) than the sifted sand.

Table 1 also shows that the two filling techniques produce differences in the cohesion strength of the materials. At peak friction, the cohesion of the sifted sand ranges between 57.3 Pa and
angles (\(\phi\)) and coefficients (\(\mu\)) of internal friction, linearly extrapolated peak cohesion (\(C\)), and ratio of strain softening (expressed as a percent) obtained with a ring-shear tester. (a) Sand, using different heights, test cell filling techniques (sifting and pouring) and granulometries; (b) sand mixtures, using only the pouring technique; (c) angles (\(\phi_b\)) and coefficients (\(\mu_b\)) of the peak basal friction of the sand.

Table 1(b) provides the results of the frictional properties of the sand mixtures. The values of the peak (\(\phi\)) internal friction are similar among the mixtures. Note that the peak and stable internal friction values are nearly identical for the sand barite and sand mica mixtures. Consequently, both mixtures present a low strain softening rate (7%), which is much lower than that of the sand (32% and 18% for sand poured from 10 cm and from the edge of the cell, respectively (Table 1(a) and (b))). Moreover, the reactivation peak friction exhibits slightly higher values than the peak friction. According to Krantz (1991), the increased internal reactivation peak strength may be related to strain hardening, which is produced by the increased compaction in sand mixtures. Thus, in our sand mixtures, the powdery barite and the fine mica crystals may have caused strain hardening during reactivation.

![Graph a](image1.png) ![Graph b](image2.png)

**Fig. 1.** Shear stress plotted as a function of time for the tested sands in the ring-shear tester; (a) sifted and (b) poured sand, both from a height of 20 cm.
2.2.3. Comparison of sand and sand mixtures

The comparison of the peak friction values of sand and sand mixtures does not reveal any substantial change. The sand and the sand barite mixture poured from a height of 10 cm present a difference of 1.33°, whereas the sand and the sand mica mixture poured from the edge of the cell exhibit a variation of 1.61°.

However, significant differences are seen in the stress—time relationships and in the peak strength. From Fig. 2, it is clear that the time interval (i.e., the amount of shear strain) between the onset of shear strain and failure is lowest for sand (<100 s), intermediate for the sand barite mixture (ranging from 100 s to 200 s) and highest for the sand mica mixture (close to 200 s). In the literature, this interval is referred to as a diffuse deformation (Lohrmann et al., 2003; Panien et al., 2006), and it is associated with sample compaction/decompaction during pre-failure deformation. The peak strength (i.e., failure) corresponds to the maximum compaction rate after a period of plastic strain hardening.

The diagram of the peak shear stress as a function of the normal stress, which is shown in Fig. 3, reveals that the sand mica mixture failure under a lower amount of peak shear stress than the other tested sands, although the variation of the angle of the internal friction is low.

3. Physical-analog experiments

3.1. Scaling

According to Hubbert (1937) and Ramberg (1981), to ensure dynamic similarity in the experimental models, two conditions should be satisfied: the angle of internal friction of the analog materials should be similar to that of their natural counterparts, and the models should obey the stress ratio \( \frac{\sigma^*}{\sigma^*} = \rho^* \times g^* \times l^* \), where \( \sigma^* \), \( \rho^* \), \( g^* \) and \( l^* \) are the ratios of cohesion (which have the dimension of stress), density, gravitational acceleration on Earth and length, respectively, between the model and nature.

Based on our study, the angle of internal friction (at peak strength) varies between 36.02° and 39.25° for the poured sand and sand mixtures, which justifies their use to simulate rocks of the upper crust with angles of internal friction that vary from 31° to 41° (Handin, 1966; Byerlee, 1978; Jaeger and Cook, 1979).

Table 3 depicts values of \( \rho^* \) that range from 0.34 to 0.66 and \( g^* = 1 \) for a length ratio \( l^* = 10^{-5} \) (thus, 1 cm in our experiments corresponds to 1 km in nature). To ensure dynamic similarity, \( \sigma^* \) and \( l^* \) must have similar values. In this study, similarity indicates that the stress ratio varies from \( 3.4 \times 10^{-6} \) to \( 6.6 \times 10^{-6} \). Because sedimentary rocks exhibit a cohesion that varies between 10 MPa and 20 MPa (Handin, 1966), the cohesion of the sand used in our laboratory must be between 34 Pa and 66 Pa. These values are significantly less than those measured by the RST-XS (between 57 Pa and 188 Pa). The difference may be related to the uncertainties that arise from the linear extrapolation of the Coulomb envelope under low normal stresses (Schellart, 2000; Panien et al., 2006).

3.2. Materials and methods

The analog models of the sand and the two sand mixtures (Table 2) were set up on the same substrate (a sheet of cardboard) in glass-sided boxes with equal internal dimensions and a basal dip of 0°. The models had a length of 35 cm, a width of 20 cm and a total thickness of 3 cm and were deformed by the displacement of the mobile back wall (Fig. 4). To enable observations of the internal wedge structure, thin horizons of differently colored sands were introduced into the models. In all of the models, deposition was initiated with the same sand layer; hence, the angle of basal friction (\( \Phi_b = 29.17° \)) was the same for all experiments (Table 1(c)).

The deposition of the analog material was implemented manually and poured from a height of 10 cm (corresponding to the pouring height of the ring-shear tests for sand and the sand barite mixture). To prevent variations in thickness, the analog material was slightly compacted after the deposition of each 0.5-cm-thick layer with gentle pats made with a spoon. In the ring-shear tests, the normal stress variation, from 400 to 2400 Pa, modified the peak shear strength by a maximum of 200 Pa. Thus, our gentle compaction procedure in the physical models cannot substantially modify the frictional properties. Gomes and Caldeira (2011) have

![Fig. 2. Shear stress plotted as a function of time, with the stress—strain curves of the analog materials: (a) sand and (b) sand barite mixture poured from a height of 10 cm; (c) sand and (d) sand mica mixture poured from the edge of the annular cell.](image-url)
shown that this procedure causes a negligible change in the angle of the internal friction of the sand.

All experiments were subjected to a shortening of 12 cm (34.3%) at a constant velocity of 2.5 cm/h (0.69 × 10⁻³ cm/s). At regular shortening intervals (1 cm), measurements of the wedge slope angle and wedge elevation were made at the transparent sidewalls, and the cross-sectional evolution was photographed. In addition, the amount of displacement at the initiation of the forward thrust was measured. Because of a lack of accuracy along the sidewalls, the fault dip angles were not measured. At the end of the experiments, the models were wetted and sectioned. To ensure reproducibility, each experiment was performed twice.

Lateral friction between the sand and the glass sides cannot be avoided; however, the impact of this phenomenon should be nearly the same in all three of the performed experiments. The friction caused a slightly smaller deformation at the sidewalls than at the wedge center. To show this fact, we present sections cut through the middle of the model domain (Fig. 5).

3.3. Description of the experiments

The cross-sectional structural evolution of our three models after 1, 2, 4, 8 and 12 cm of mobile wall displacement is shown along the sidewalls in Fig. 4. The shortening in our models is similar to that described in several previous studies (e.g., Mulugueta and Koyi, 1987; Huiqi et al., 1992). In front of the rigid mobile back wall, a wedge of piggyback imbricate stacks was formed by serial repetition of sand pack sliding along the basal detachment, layer-parallel shortening and subsequent faulting. During the progressive deformation, no reactivation of older faults was recognized, and the formation of back thrusts was not a common feature. Back thrusts were only clear in the model center of the quartz–barite mixture model (Fig. 5(b)). In the following section, the kinematics of the faults and deformation styles of the different analog materials are described.

3.3.1. Kinematics of the faults

The most striking observation from the sand experiment (for ease of description, this experiment is also called experiment 1 or model 1) is the strong steepening (up to 90°) of the lower F1-fault segment, which occurs even before a new foreland-vergent thrust has nucleated in the footwall (Fig. 4(a)). Throughout the progressive deformation, the younger faults, F2 and F3 (F2A does not extend along the strike) exhibited a similar pattern of steepening that led to a strong convex-upward listric thrust geometry. The forward migration of the zone of shortening generated in the hinterland vertical ejection of the sand, which was progressively confined between the lower fault segments and the mobile back wall. The older thrusts became progressively inactive as the slip stopped growing, and a piggyback anticlinal stack was generated.

We suggest that the steepening of the lower fault segments is a consequence of the mechanical behavior of our sand, which enables vertical grain flow in the hinterland, while slight layer-parallel shortening occurs in the foreland. According to Koyi (1995), who used bed length balancing for two single layers at different stratigraphic levels in sand models, layer-parallel shortening is greater in the deep layers than in the shallow layers. This layer-parallel shortening causes a buttress effect in the deepest layer and, thus, steepening of only the lower segments of thrust faults.

A similar pattern of thrust steepening of the lower fault segments is only poorly visible in the quartz–barite mixture experiment (experiment 2), and it does not occur in the quartz–mica experiment (experiment 3). During the progressive deformation, experiment 2 also produced a differential counterclockwise rotation of the lower segments of the thrusts. However, the fault

Table 2

<table>
<thead>
<tr>
<th>Analog material</th>
<th>Composition</th>
<th>Bulk density (kg/m³)</th>
<th>Sand granulometry (µm)</th>
<th>Sand mixtures granulometry (µm)</th>
<th>Shape of sand or of the mixed products</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pure sand</td>
<td>SiO₂ – 99%; Tourmaline, micas, opaque</td>
<td>1500°</td>
<td>≤350</td>
<td>–</td>
<td>Angular</td>
</tr>
<tr>
<td></td>
<td></td>
<td>1500°</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Sand barite mixture</td>
<td>Barite: BaSO₄ – 79.4%; SiO₂ – 11.4%; Fe₂O₃ – 7.52%; other oxides</td>
<td>1810</td>
<td>≤350</td>
<td>≤210 µm</td>
<td>Angular/subangular/sub-rounded</td>
</tr>
<tr>
<td>Sand mica mixture</td>
<td>Sand and Muscovite</td>
<td>940</td>
<td>≤350</td>
<td>≤1 mm</td>
<td>Tabular</td>
</tr>
</tbody>
</table>

a Sifted sand from a height of 20 cm.
b Poured sand from a height of 20 cm.
c Poured sand from a height of 10 cm.

Fig. 3. Comparison between the failure envelopes for all sands analyzed. Note that the slope angles (the angles of internal friction) do not change much; however, the peak shear stress of the sand mica mixture is significantly lower than those of the other sands. h is the filling height of the annular cell.
rotations were more moderate, and the fault traces convexed up only slightly. Consequently, the vertical grain flow in the hinterland was less pronounced. In experiment 3, the approximately straight fault traces suggest a more homogenous behavior of the quartz–mica mixture in that the layer-parallel shortening was not as great in the deep layers as it was in the sand. Consequently, counter-clockwise rotations affected the entire F1, F2, F3 and F4 thrust traces.

In the quartz–barite mixture experiment, only three thrusts were formed (Fig. 5(b)), but on the sidewall, additional faults can be seen (e.g., fault F1A, in Fig. 4(b)(iv) and (v)). The surface structures simultaneously with a horizontal ductile strain. This result suggests that the vertical grain flow in the hinterland of the analog material in the back of the wedge occurred as indicated by the folded marker horizon. The high surface slope angles at 3–8 cm of shortening in model 1 (ranging from 47° to 37°) are the result of rapid increases in wedge elevation caused by the strong vertical ejection of sand, as described above. An abrupt decrease (of 24°) occurred because

3.3.2. Wedge elevation and wedge surface slope

The wedge elevations were determined at distances of 2 cm from the inward-moving mobile wall, and the wedge surface slopes were determined by measuring the slope of the line linking the tips of the thrust sheets (Fig. 8). From Fig. 8, which compares the convergent wedges of the three experiments, it is clear that the elevation of the wedges varied significantly after a 12 cm displacement. The final surface slopes (α) are equal for models 1 and 3, with α = 22°, but vary significantly compared to model 2, with α = 17°.

A progressive increase in the wedge elevation occurred during deformation in the three experiments (Fig. 9). The highest values correspond always to model 1, whereas the smallest value corresponds to model 2. At the end of the deformation, experiments 1, 2 and 3 presented elevation values of 5.6 cm, 3.5 cm and 4.9 cm, respectively. The thickening observed in experiments 1 and 2 is a consequence of the distributed vertical grain flow of the frictional sand in the back of the wedges. In experiment 3, an additional horizontal strain accommodation of the sand mica mixture occurred as indicated by the folded marker horizon.

The variation of the surface slopes of the wedges is given in Fig. 10. The high surface slope angles at 3–8 cm of shortening in model 1 (ranging from 47° to 37°) are the result of rapid increases in wedge elevation caused by the strong vertical ejection of sand, as described above. An abrupt decrease (of 24°) occurred because

Table 3

<table>
<thead>
<tr>
<th>Parameter</th>
<th>Model</th>
<th>Nature</th>
<th>Ratio model/nature</th>
</tr>
</thead>
<tbody>
<tr>
<td>Length</td>
<td>0.01 m</td>
<td>10^3 m</td>
<td>( r^4 \times 10^{-3} )</td>
</tr>
<tr>
<td>Density</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>- Pure sand</td>
<td>1480–1580 kg/m^3</td>
<td>2400–2800 kg/m^3</td>
<td>( \rho^* = 0.52–0.66 )</td>
</tr>
<tr>
<td>- Sand barite mixture</td>
<td>1810 kg/m^3</td>
<td>2400–2800 kg/m^3</td>
<td>( \rho^* = 0.75–0.65 )</td>
</tr>
<tr>
<td>- Sand mica mixture</td>
<td>940 kg/m^3</td>
<td>2400–2800 kg/m^3</td>
<td>( \rho^* = 0.39–0.34 )</td>
</tr>
<tr>
<td>Gravity</td>
<td>9.8 m/s^2</td>
<td>9.8 m/s^2</td>
<td>( g^* = 1 )</td>
</tr>
<tr>
<td>Stress (cohesion) (( \sigma = \rho g ))</td>
<td></td>
<td></td>
<td>( \sigma^* = 3.4 \times 10^{-6} ) to ( 6.6 \times 10^{-6} )</td>
</tr>
</tbody>
</table>

Fig. 4. Five representative photographs of the progressive deformation of the three experiments, taken through the transparent sidewalls; (a) sand, (b) sand barite mixture and (c) sand mica mixture (length values indicate the magnitude of shortening). F1, F2, F3 and F4 – thrust faults; F1A, F2A and F3A – thrust faults with restricted continuity in 3D, which are represented in the profile by dashed lines.
of the formation of the youngest fault, F4, which had a higher thrust space than the older ones. From that point onward, the slope angle of the wedge only grew slightly because of the small displacement along F4. In experiment 2, the sand barite mixture model also exhibited an oscillation in its surface slope angles, but the values were much smaller (varying from $29^\circ$ to $17^\circ$) than in model 1. Only experiment 3 showed relatively regular surface slope angles (that ranged from $26^\circ$ to $23^\circ$), despite the high thrust space between faults F3 and F3A (Figs. 4 and 7).

4. Summary and discussion

4.1. Frictional properties

Our ring-shear tests were consistent with those performed by Lohrmann et al. (2003) and Panien et al. (2006), in that the sifted and poured sands exhibited different rheological behaviors. According to these authors, sifting produces more densely packed sand because it produces better grain fit than pouring. Consequently, sifted sand is associated with a higher bulk density (see also our Table 2), higher compaction and greater strain hardening preceding failure during shear testing than that of poured sand.

Our three sands have the following common features: (1) an elastic/frictional plastic mechanical behavior and (2) a peak friction (with a maximum variation of 2.06° between sands handled with the same filling technique, or 3.23° without considering the filling technique).

The main differences between the analyzed sands occur in the diffuse deformation interval and in the shear strength. The latter is similar for sands and the sand barite mixture, but it is lower for the sand mica mixture (Fig. 3).

According to Panien et al. (2006), the diffuse deformation in the ring-shear tests is influenced by both the handling techniques and the type of granular material, specifically the grain shape and grain-size distribution. Angular and subangular shapes and heterogeneous grain-size distributions demand a longer diffuse deformation because of the higher dispersion of stresses. This effect may explain the larger amount of diffuse deformation that was observed for the sand mixtures compared to the sand and, thus, their higher amount of shear strain to reach failure. Similar conclusions can be deduced from the results of our analog models, in which fault generation took longer for the two mixtures than for the sand.

4.2. Analog models

Our convergent sandbox experiments with sand and sand mixtures show a similar first-order evolution, but they differ significantly in detail. Even flow maps have not been presented of our three models, the above model descriptions combined with the ring-shear test results allow for correlations between the different features to the types of deformation. Thus, we suggest that among our three experiments, the sand mica mixture of model 3, which had the more homogenous grain flows, shows the strongest distributed deformation, that is, the more plastic behavior, for a number of reasons. First, in the ring-shear tests, the sand mica mixture produced the highest amount of diffuse deformation (the most gentle strain–stress curves) and the lowest amount of shear strength, which delayed thrust formation in the analog experiments. Second, in the thrust's footwall layer, parallel shortening affected the deep and shallow layers in the same way, and in the thrust's hanging wall, the strain was accommodated by both vertical and horizontal grain flow (see the F1 hanging wall of the sand mica mixture in Fig. 5 compared to the F1 hanging wall of the
The homogenous variation in the evolution of the critical wedge slope (of $3^\circ$) of the sand mica mixture during deformation (Fig. 10) may be the fourth argument for its strong distributed deformation.

Our sand model revealed a strong oscillation of the critical wedge slope, similar to the observations by Mulugeta and Koyi (1992) (with oscillations between $10^\circ$ and $20^\circ$). However, comparisons of critical wedge slope evolutions are somewhat troublesome for models (with different grain shapes and sizes), which made it possible to demonstrate the influence of these variations on the kinematics and the geometry of the convergent systems.

The comparison of our three experiments suggests that more plastic granular materials produce a more homogenous thrust fault behavior (with straight traces) than low-plasticity materials, leading to a homogenous evolution of the wedge slope. Comparisons among the sand model and the sand mica mixture model show that, despite similarities in their peak friction (here, we do not consider cohesion because of the difficulty in determining satisfactory values, as described above), models with the same basal friction may produce distinct peak strengths. Consequently, they may be characterized by different rheological behaviors. Finally, our models suggest that when the angle of basal friction is the same in all experiments, the wedge height depends on the ability of the vertical grain flow of each analog material.

Now, the question is, are the materials suitable for simulating different competent rock units in the upper crust? Their suitability certainly depends on the deformation features being investigated. Because of insufficient knowledge of the associated rheology, shear strength and an infinite variety of other deformation processes of rocks found in nature, it is impossible to search for ideal analog materials. However, physical models allow us to discuss general similarities. For comparable conditions of pressure and temperature, it is reasonable to assume that low-porosity basement rocks (e.g., migmatites and gneisses) may have higher peak strengths than...
supracrustal rocks or basin infills (e.g., schists, quartzites, phyllites or clastic and chemical sedimentary rocks). Among these rock types, the latter may constitute the less competent unit because of its composition and anisotropies. Thus, we suggest that the lower shear strength of the sand mica mixture renders this material to be more suitable for simulating supracrustal rocks in thick-skinned tectonics. With regards to the sand and the sand barite mixture, the former presented less ductile features during our shortening experiments, indicating that sand is the most appropriate material to use for replicating competent rock units. In extensional systems, the sand mica mixture may be used to simulate syn-rift deposits, and sand may be used to simulate the pre-rift blocks.

Acknowledgments

The author wishes to express her thanks to Petrobrás for funding the project, which made the acquisition of the ring-shear tester (RST-XS) possible, and to the Federal University of Ouro Preto, which provided financial support for language editing. Special thanks to Hemin Koyi and Fernando Ornelas Marques for the detailed, constructive comments that improved the manuscript.

References


Fig. 10. Plots of wedge surface slope as a function of shortening for the three experiments.

<table>
<thead>
<tr>
<th>Shortening (cm)</th>
<th>0</th>
<th>10</th>
<th>20</th>
<th>30</th>
<th>40</th>
</tr>
</thead>
<tbody>
<tr>
<td>Wedge-Surface Slope (%)</td>
<td>0</td>
<td>20</td>
<td>40</td>
<td>60</td>
<td>80</td>
</tr>
</tbody>
</table>

The sand barite mixture renders the material to be more suitable for simulating supracrustal rocks in thick-skinned tectonics. With regards to the sand and the sand mica mixture, the former presented less ductile features during our shortening experiments, indicating that sand is the most appropriate material to use for replicating competent rock units. In extensional systems, the sand mica mixture may be used to simulate syn-rift deposits, and sand may be used to simulate the pre-rift blocks.